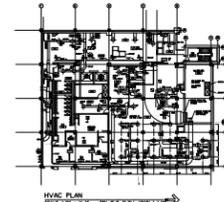
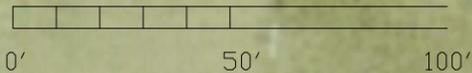






SURVEY AND CONTOURS IN FEET.  
CLOSE TO UTM GRID AND DATUM.  
CONVERSION FACTOR 3.28084



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NOTES

01 DO NOT USE THIS DRAWING FOR CONSTRUCTION UNLESS THE WORDS 'CONSTRUCTION SET' APPEARS IN THE REVISIONS COLUMN

PRELIMINARY ISSUE -  
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REV #	YYMMDD
REVISIONS	A

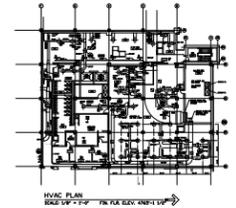
VIRGIN GORDA  
BASKETBALL COURT

TOPOGRAPHICAL

DATE: MAY 2012	DRAWN: IH
SCALE: N.T.S	DWG #: S100

PLAN : --

SCALE : N.T.S



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PRELIMINARY ISSUE - NOT FOR CONSTRUCTION.

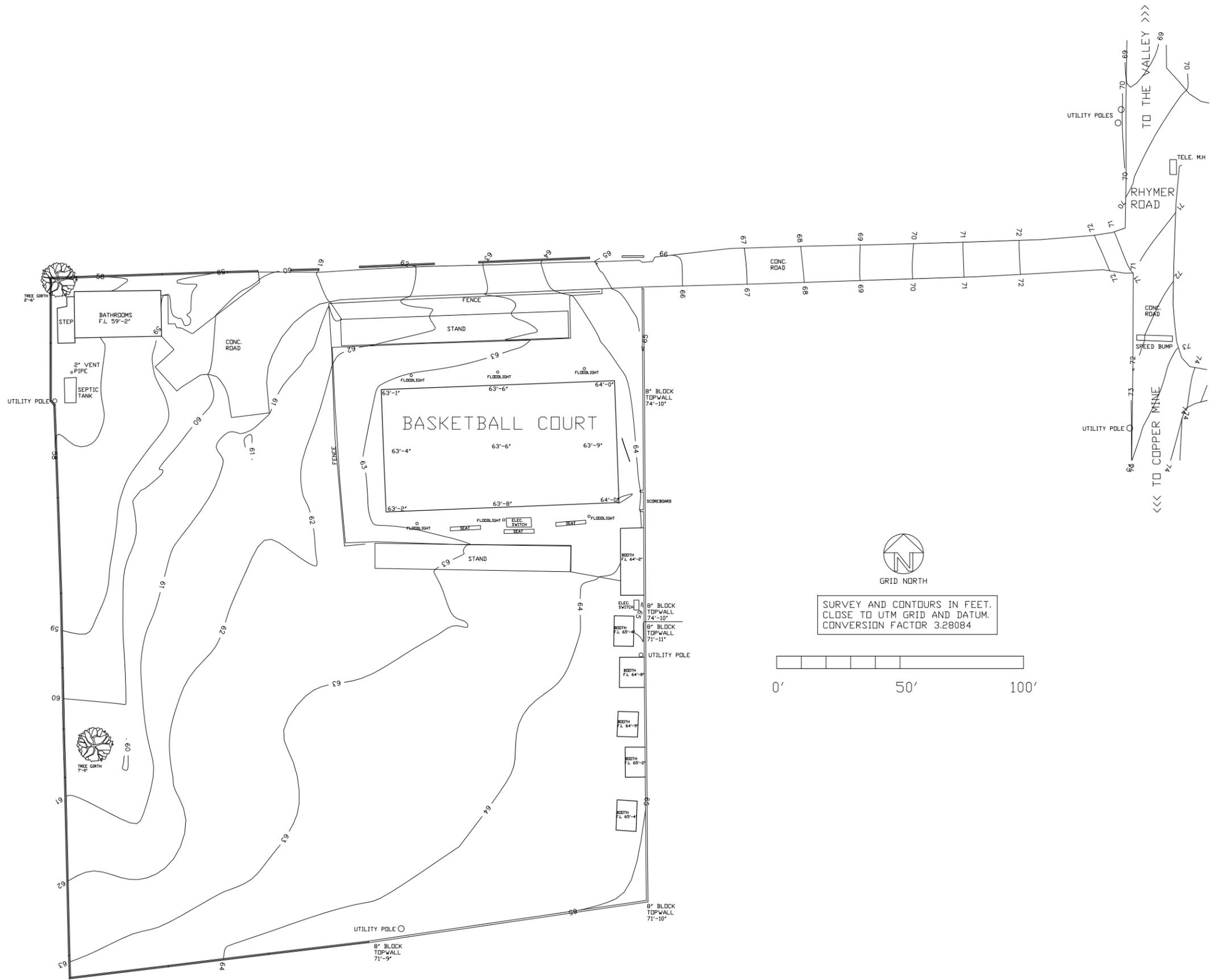
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REVISIONS	A
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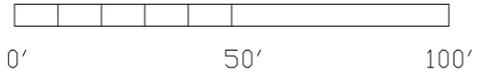
**VIRGIN GORDA  
 BASKETBALL COURT**

**TOPOGRAPHICAL**

DATE: MAY 2012	DRAWN: IH
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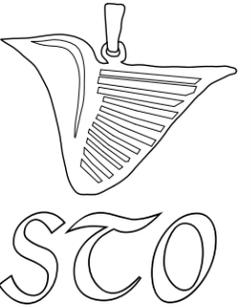


GRID NORTH  
 SURVEY AND CONTOURS IN FEET.  
 CLOSE TO UTM GRID AND DATUM.  
 CONVERSION FACTOR 3.28084



PLAN : --

SCALE : N.T.S



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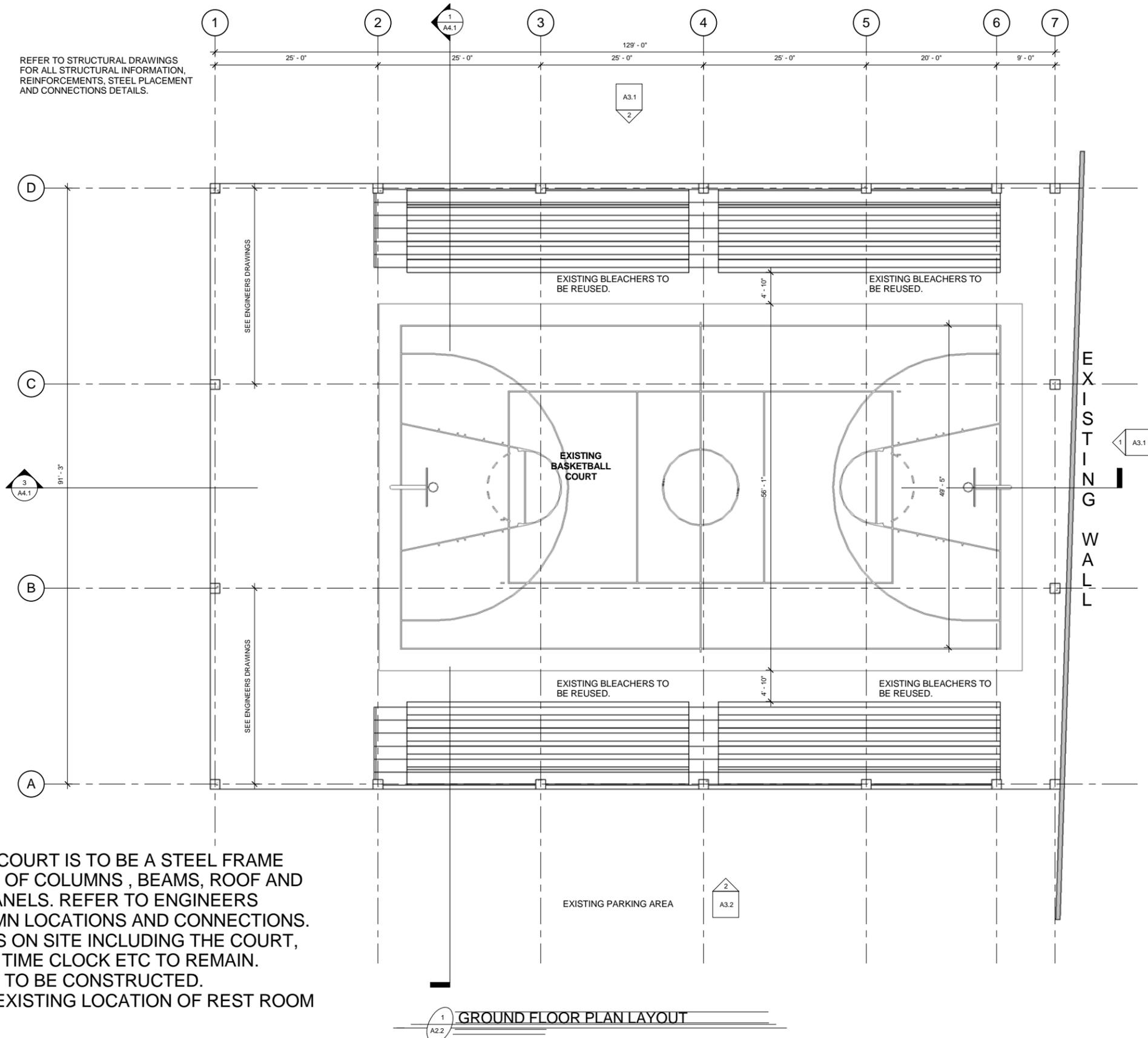
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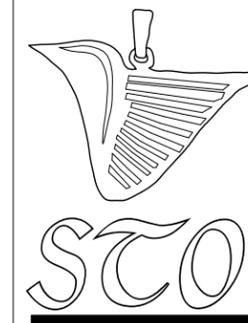
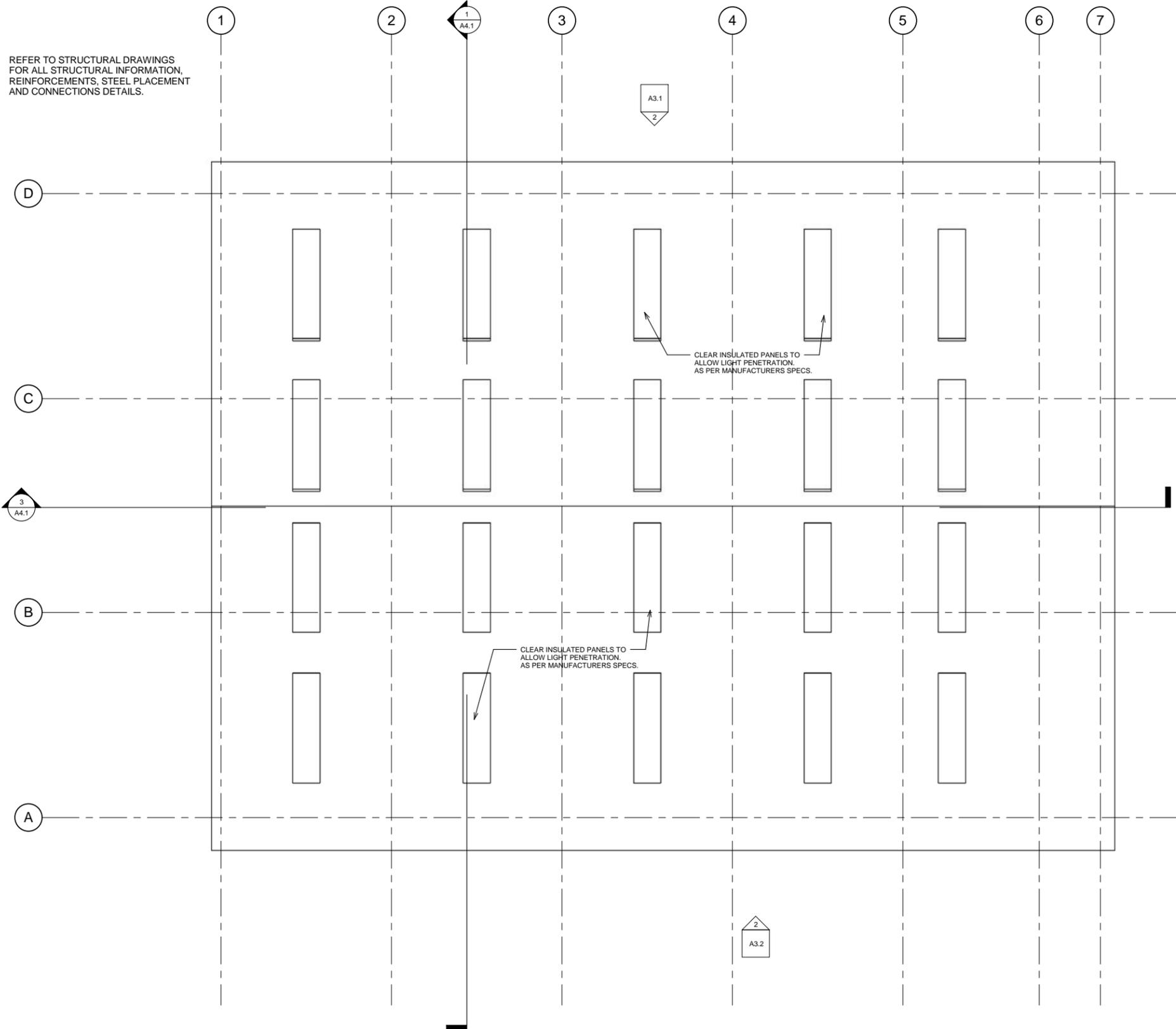
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	R.1		SCALE DATE	1/8" = 1'-0" JUNE 2012
DRAWING	FIRST FLOOR PLAN LAYOUT		COPPER MINE ROAD, THE VALLEY VG, BVI	



**NOTE**  
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1 A2.2 **GROUND FLOOR PLAN LAYOUT**

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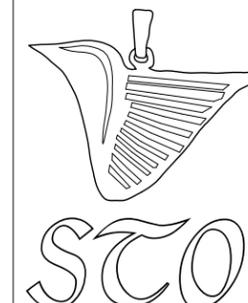
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DRAWING #	A2.4	DRAWN BY JUNE 2012	CHECKER Author
	PROJECT		
DRAWING	ROOF PLAN LAYOUT	COPPER MINE ROAD, THE VALLEY VG, BVI	

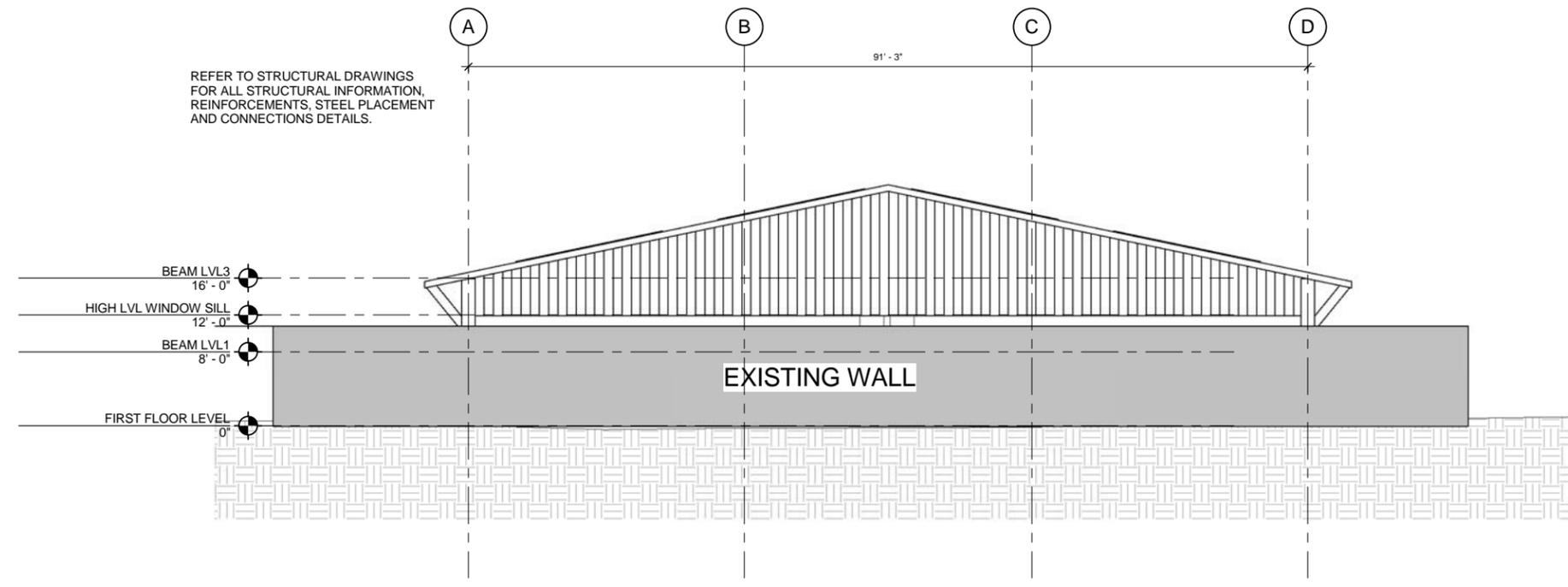
**1** ROOF PLAN LAYOUT  
 A2.4



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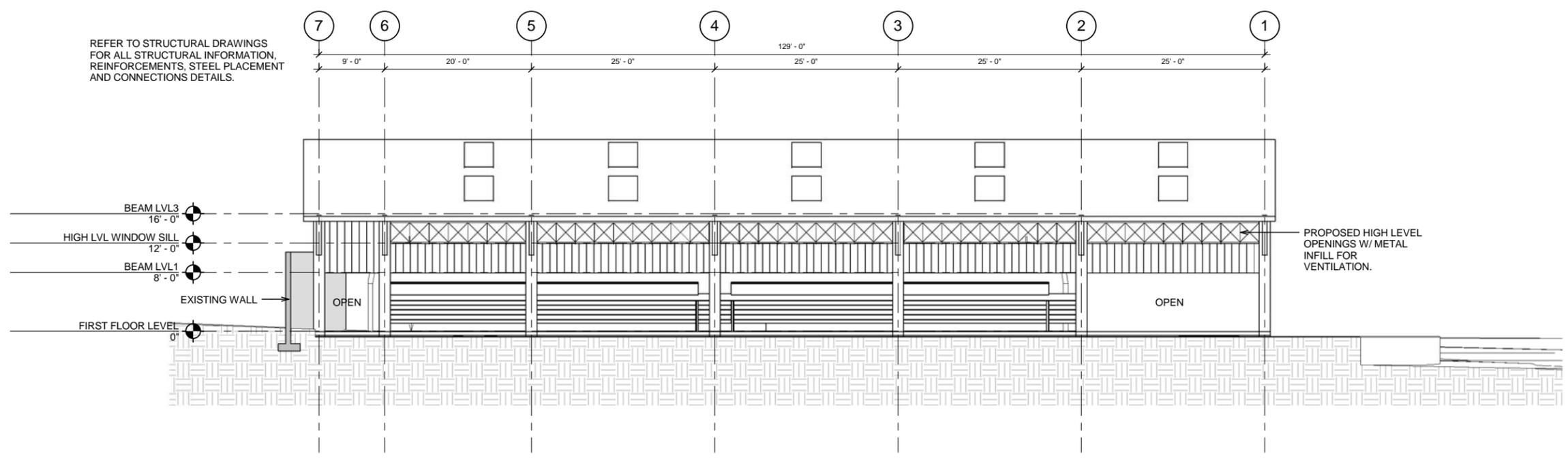
REFER TO STRUCTURAL DRAWINGS FOR ALL STRUCTURAL INFORMATION, REINFORCEMENTS, STEEL PLACEMENT AND CONNECTIONS DETAILS.



1 EAST ELEVATION  
 A3.1 1/8" = 1'-0"

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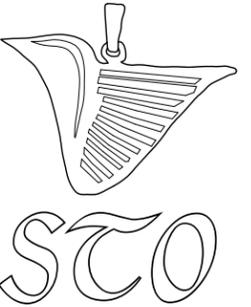


2 NORTH ELEVATION  
 A3.1 1/8" = 1'-0"

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DRAWING	ELEVATIONS		COPPER MINE ROAD, THE VALLEY VG, BVI
	PROJECT	VG B-BALL COURT ENCLOSURE	
DRAWING #	A3.1	SCALE DATE	1/8" = 1'-0" JUNE 2012

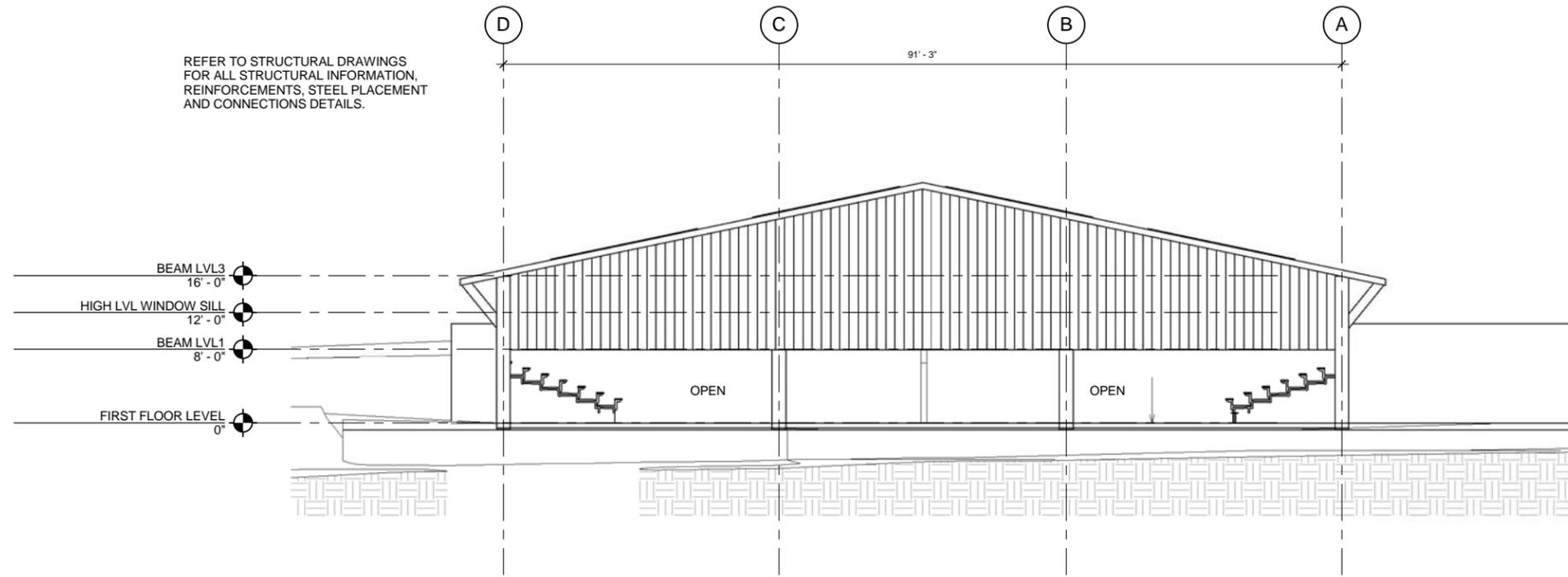


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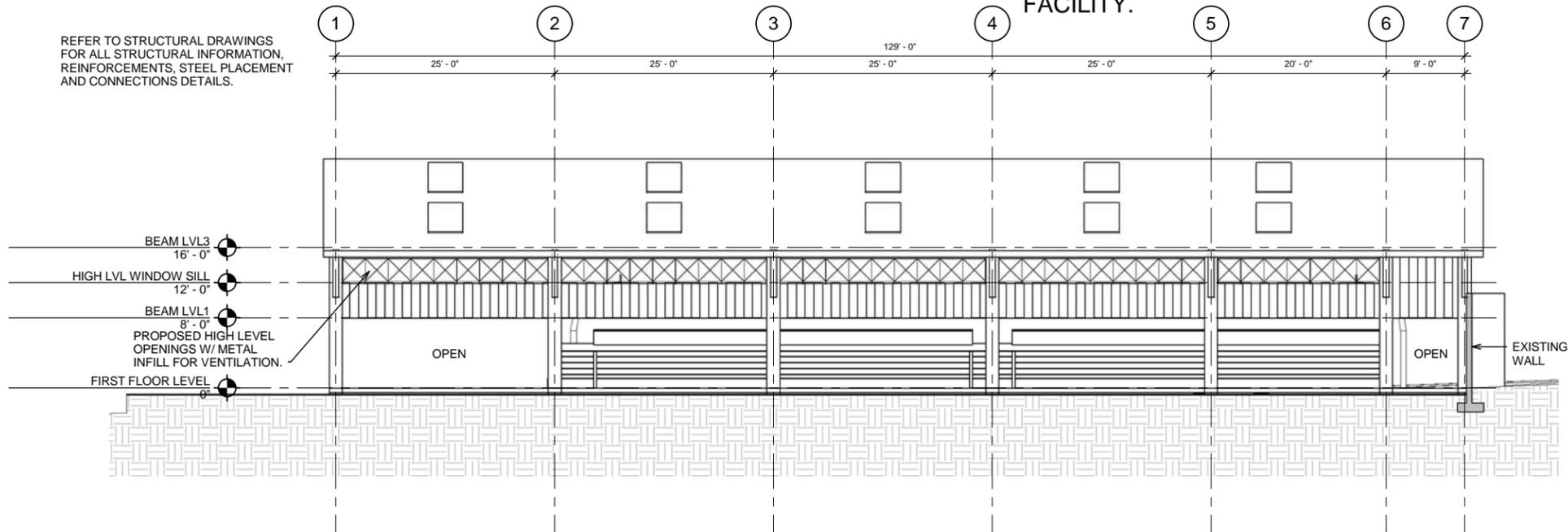
REFER TO STRUCTURAL DRAWINGS FOR ALL STRUCTURAL INFORMATION, REINFORCEMENTS, STEEL PLACEMENT AND CONNECTIONS DETAILS.



1 WEST ELEVATION  
A3.2 1/8" = 1'-0"

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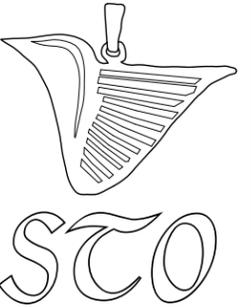


2 SOUTH ELEVATION  
A3.2 1/8" = 1'-0"

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DRAWING #	A3.2	SCALE DATE	1/8" = 1'-0" JUNE 2012	DRAWN BY CHECKER	Author
	PROJECT		VG B-BALL COURT ENCLOSURE		DRAWING #
DRAWING		ELEVATIONS	COPPER MINE ROAD, THE VALLEY VG, BVI		

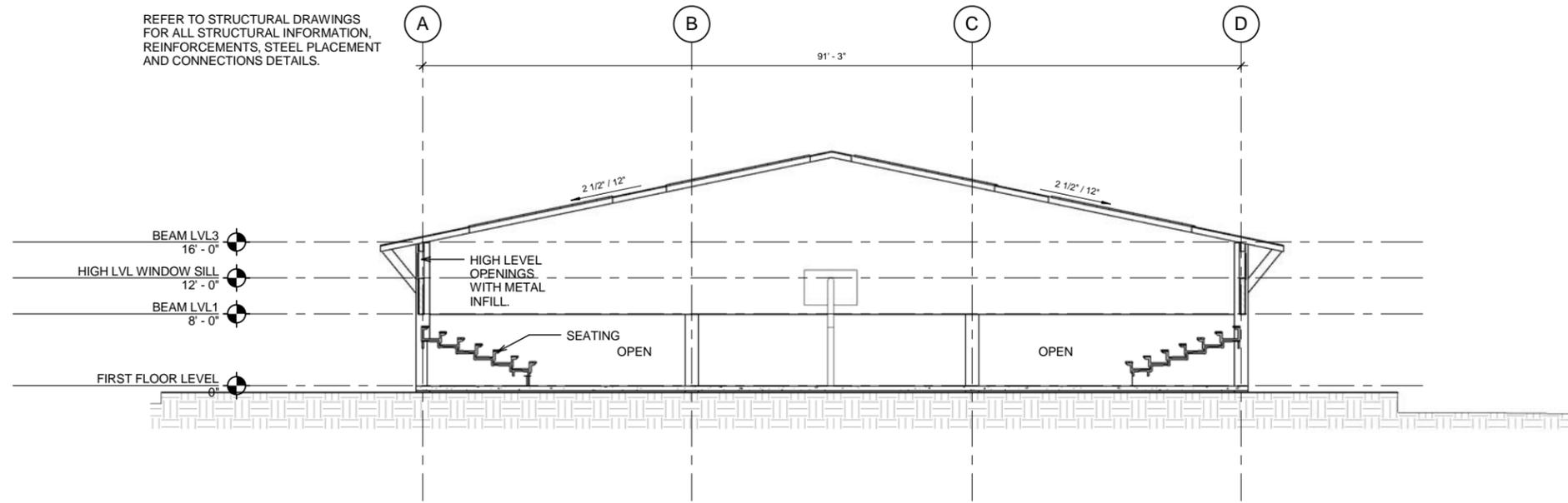


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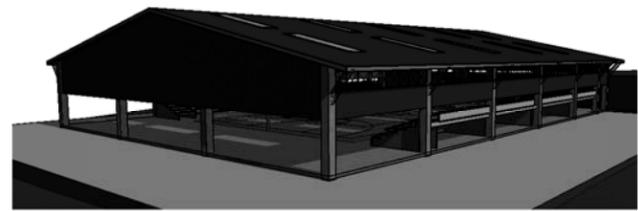
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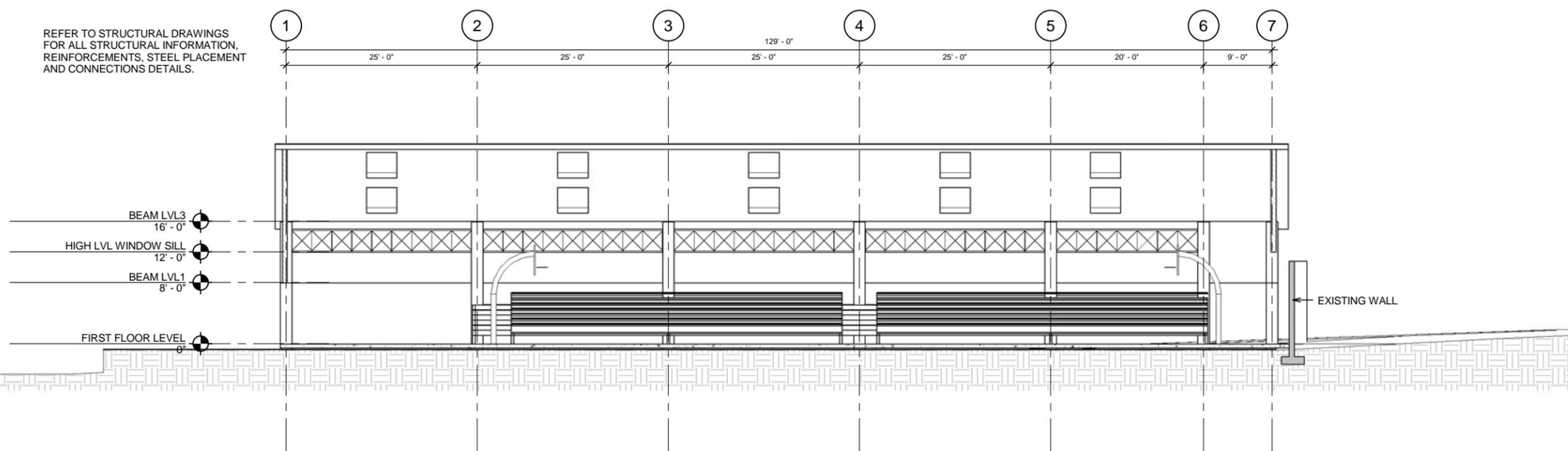
1 CROSS SECTION A-A  
A4.1 1/8" = 1'-0"



2 3D View 2  
A4.1

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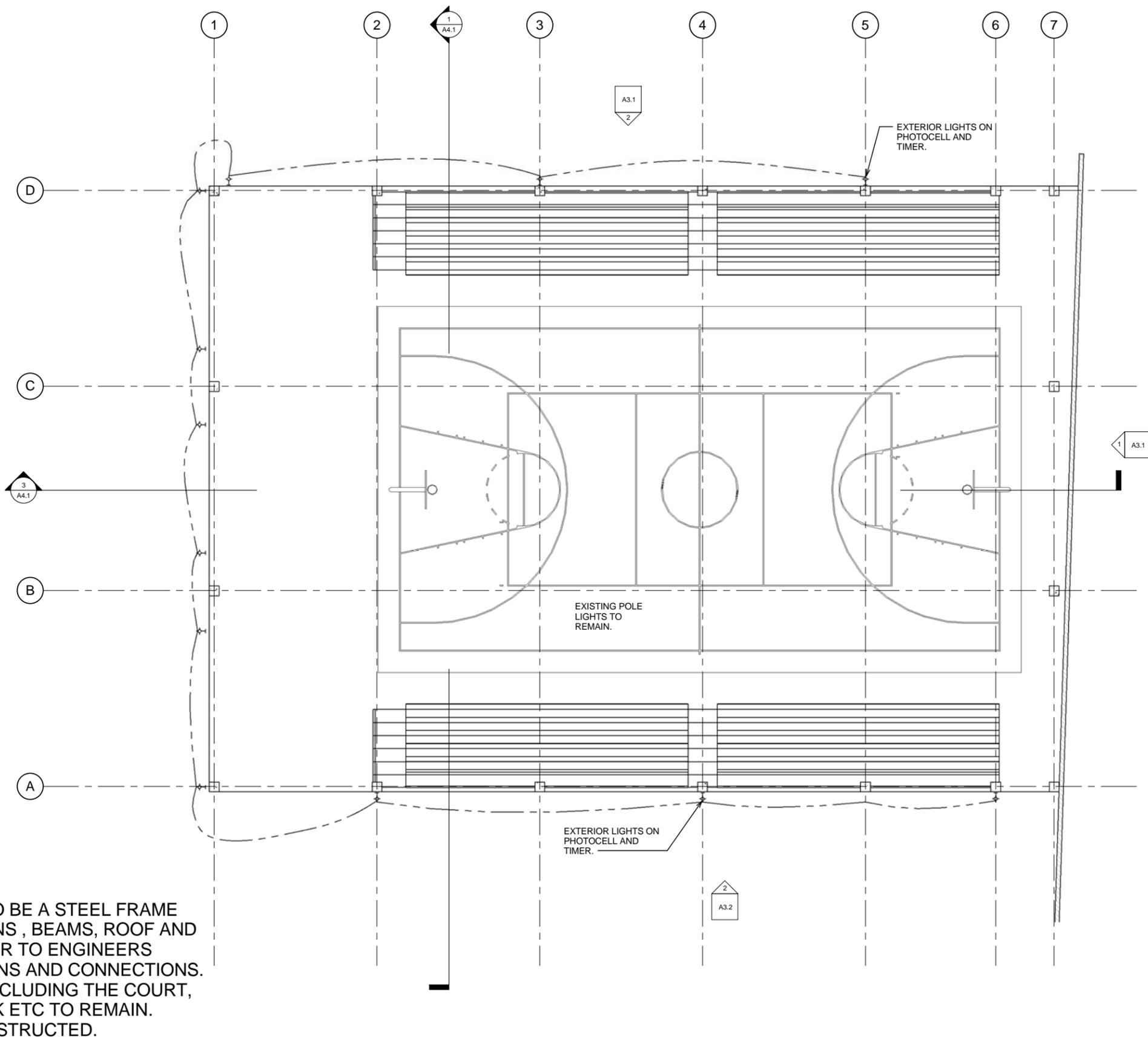
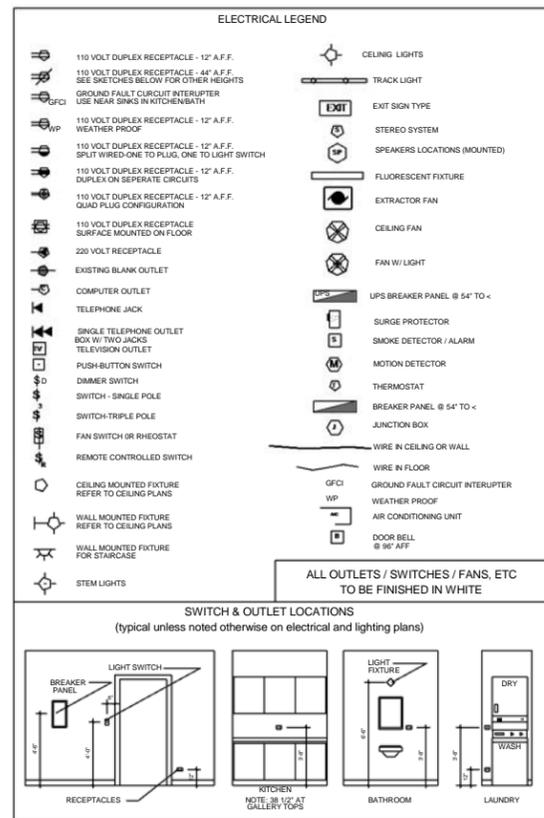


3 LONGITUDINAL SECTION B-B  
A4.1 1/8" = 1'-0"

No.	Date	Description

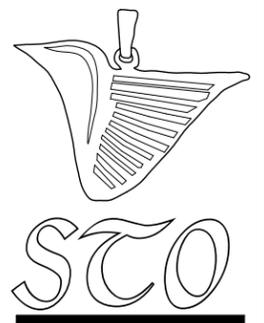
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DRAWING	BUILDING SECTION A-A & B-B		DRAWN BY S. BROWN	DATE JUNE 2012	SCALE 1/8" = 1'-0"	PROJECT	VG B-BALL COURT ENCLOSURE
	DRAWING						D. STOUTT
DRAWING #	A4.1		DATE		SCALE		R.1



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**GROUND FLOOR ELECTRICAL PLAN**  
 1/8" = 1'-0"



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DRAWING #	E2.1	PROJECT	VG B-BALL COURT ENCLOSURE		DRAWN BY As indicated JUNE 2012	CHECKER Author	
	R.1		GROUND FLOOR ELECTRICAL				
DRAWING #	E2.1	PROJECT	VG B-BALL COURT ENCLOSURE	DRAWN BY	As indicated JUNE 2012	CHECKER	Author
DRAWING #	E2.1	PROJECT	VG B-BALL COURT ENCLOSURE	DRAWING	GROUND FLOOR ELECTRICAL	CHECKER	Author

# STRUCTURAL GENERAL NOTES

### 1.0 LIVE LOADS USED IN DESIGN

1.1 ROOF LIVE 20 PSF  
 1.2 TYPICAL FLOOR 40 PSF  
 1.3 WIND DESIGN  
 1.3.1 WIND VELOCITY 159 MPH V 3S  
 1.3.2 WIND EXPOSURE C  
 1.4 EARTHQUAKE ZONE 4 (UBC REFERENCE)  
 1.4.1 SEISMIC DESIGN CATEGORY D  
 1.4.2 SITE CLASS C/D  
 1.4.3 SPECTRAL RESPONSE ACCELERATIONS  
     S<sub>s</sub> 1.54, S<sub>1</sub> 0.46  
 1.4.4 SPECTRAL RESPONSE COEFFICIENTS  
     S<sub>ds</sub> 0.933, S<sub>d1</sub> 0.439  
 1.5 FLOOD DATA: SEE SPECIFIC SITE NOTE

### 2.0 FOUNDATIONS

2.1 MAXIMUM SOIL DESIGN PRESSURE 2000 PSF  
 2.2 ALL FOOTINGS ARE TO BE PLACED ON FIRM, UNDISTURBED NATURAL GROUND, APPROVED BY THE ENGINEER. SEE SPECIFICATION FOR INFORMATION ON PILED FOUNDATIONS.  
 2.3 DO NOT PLACE BACKFILL AGAINST BASEMENT WALLS UNTIL BASEMENT AND FIRST FLOORS ARE IN PLACE.  
 2.4 CENTER ALL FOOTINGS UNDER WALLS, COLUMNS OR GRID LINES UNLESS OTHERWISE NOTED IN THE DRAWINGS

### 3.0 CONCRETE

3.1 ALL CAST-IN-PLACE CONCRETE SHALL BE MADE WITH TYPE-2 PORTLAND CEMENT, STONE AGGREGATE AND SHALL DEVELOP 3000 PSI COMPRESSIVE STRENGTH IN 28-DAYS.  
 3.2 SLABS, TOPPING, FOOTINGS BEAMS AND WALLS SHALL NOT HAVE JOINTS IN HORIZONTAL PLANE. ANY STOP IN CONCRETE WORK MUST BE MADE AT CENTER OF SPAN WITH VERTICAL BULKHEADS AND SHEAR KEYS, UNLESS OTHERWISE SHOWN. ALL CONSTRUCTION JOINTS SHALL BE AS DETAILED OR AS APPROVED BY THE ENGINEER. SEE NOTE 4.0 BELOW.  
 3.3 ALL CONCRETE WORK AND REINFORCEMENT DETAILING SHALL BE IN ACCORDANCE WITH ACI BUILDING CODE 318-11. ALL EXPOSED EDGES OF CONCRETE WORK SHALL HAVE 1/2" MIN. CHAMFER. USE STANDARD HOOKS ON DOWELS UNLESS OTHERWISE NOTED.  
 3.4 MIXING, PLACING AND CURING OF ALL CONCRETE TO BE BY THE RECOMMENDATIONS IN ACI 305R-10, HOT WEATHER CONCRETING.  
 3.5 ALL CONCRETE SHALL BE MADE WITH GRACE DCI S CORROSION PROTECTING ADMIXTURE (EXCEPT CISTERN WALLS AND CISTERN GROUND SLABS) IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS.  
 3.6 PLACE JOINTS IN SLABS ON GRADE TO CREATE AREAS NO GREATER THAN 400 SQ. FT. COORDINATE JOINT LOCATIONS W/ ARCHITECT TO MINIMIZE IMPACT ON FLOOR FINISH. SAW CUTS CAN BE USED BUT MUST BE MADE WITHIN 3 HOURS OF PLACEMENT OF THE SLAB.

### 4.0 REINFORCEMENT

4.1 ALL REINFORCEMENT SHALL BE HIGH STRENGTH DEFORMED BARS CONFORMING TO ASTM A615-12 GRADE 60, EXCEPT TIES, STIRRUPS AND PLATE ANCHORS, WHICH SHALL BE ASTM DESIGNATION A615 / A615M - 12, GRADE 40 STEEL.  
 4.2 WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185-07 AND SHALL BE LAPPED ONE FULL MESH AT SIDES AND END SPLICES AND WIRE TOGETHER.  
 4.3 CONCRETE PROTECTION FOR REINFORCEMENT (REBAR-COVER)  
 4.3.1 CONCRETE CAST AGAINST EARTH AND PERMANENTLY EXPOSED TO EARTH 3"  
 4.3.2 CONCRETE CAST IN FORMS AND EXPOSED TO EARTH OR WEATHER 2"  
 4.3.3 CONCRETE NOT EXPOSED TO EARTH OR WEATHER SLABS AND WALLS 1-1/2"  
       BEAMS AND COLUMNS - 1-1/2"  
       PRIMARY REINFORCEMENT, TIES, STIRRUPS AND SPIRALS 1-1/2"  
 4.4 ALL BAR LENGTHS ARE DRAWN TO SCALE UNLESS OTHERWISE NOTED. NO SPLICES OF REINFORCEMENT SHALL BE MADE EXCEPT AS DETAILED OR AUTHORIZED BY THE ENGINEER. LAP SPLICES WHERE PERMITTED SHALL BE A MINIMUM OF 48 BAR DIAMETERS. MAKE ALL BARS CONTINUOUS AROUND CORNERS AND ALL RECTANGULAR OPENINGS IN CONCRETE.  
 4.5 PLACE #5 BAR (1-QTY PER 6" THICKNESS) WITH 2'-0" PROJECTION AROUND ALL OPENINGS IN SLABS OR WALLS. ALSO PROVIDE 1-#5 x 4'-0" DIAGONALLY AT EACH CORNER.  
 4.6 CONTINUOUS TOP AND BOTTOM BARS IN WALLS AND BEAMS SHALL BE SPICED AS FOLLOWS:  
 4.6.1 TOP BARS AT MID-SPAN  
 4.6.2 BOTTOM BARS OVER SUPPORTS  
 4.7 PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCEMENT AT POSITIONS SHOWN IN THE DRAWINGS. ALL REINFORCEMENT TO BE HELD SECURELY IN PROPER POSITION IN ACCORDANCE WITH ACI 318-11. ALL ACCESSORIES TO BE GALVANIZED.  
 4.8 ANY WELDED REINFORCING BARS SHALL CONFORM TO ASTM A706 OR A615, GRADE 60 AND ANSI / AWS D1.4-11.  
 4.9 ALL TIES AND STIRRUPS SHALL BE 135° SEISMIC HOOKS IN ACCORDANCE WITH ACI 318-11

### 5.0 MASONRY

5.1 ALL CONSTRUCTION AND DETAILS TO BE IN ACCORDANCE WITH ACI 530 AND 530.1-08  
 5.2 ALL REINFORCED CONCRETE BLOCKWORK SHALL BE CONSTRUCTED USING TWO CELL MASONRY UNITS ONLY, UNLESS OTHERWISE DETAILED. GROUT CELLS PROGRESSIVELY IN LIFTS NOT EXCEEDING 5 BLOCKS IN HEIGHT.  
 5.3 WALLS SHALL BE REINFORCED HORIZONTALLY AT 8" O.C. WITH TRUSS-TYPE REINFORCEMENT. THE WIRE SHALL CONFORM TO ASTM A82-07, STANDARD GAGE, GALVANIZED UNLESS SHOWN OTHERWISE.  
 5.4 EXTERIOR WALLS SHALL BE ALSO BE REINFORCED WITH 2-#5 BARS VERTICALLY AT WALL ENDS, CORNERS, EACH SIDE OF DOOR OR WINDOW OPENINGS AND AT NOT OVER 1-#5 AT 24" O.C TYPICALLY. REINFORCEMENT SHALL BE FULLY GROUTED IN PLACE. GROUT SHALL DEVELOP 3000 PSI IN 28-DAYS AND MEET ACI 530.1.  
 5.5 FILL ALL VOIDS AND BLOCK CELLS SOLIDLY WITH ACI 530.1 GROUT FOR A DISTANCE OF 24" BENEATH AND 16" EACH SIDE OF ALL BEAM REACTIONS OR OTHER CONCENTRATED LOADS, UNLESS OTHERWISE DETAILED.  
 5.6 PROVIDE A THICKENED CONCRETE SLAB UNDER ALL MASONRY WALLS ON SLABS. SEE ARCHITECTURAL DRAWINGS FOR LOCATION AND EXTENT OF WALLS, AND FOR OPENINGS, ALL OF WHICH REQUIRE LINTELS. SEE LINTEL SCHEDULE FOR FRAMING OVER NON-BEARING OPENINGS IF NOT SHOWN ON STRUCTURAL DRAWINGS. ALSO SEE MECHANICAL DRAWINGS FOR OPENINGS, ALL OF WHICH REQUIRE LINTELS.  
 5.7 CONCRETE BLOCK UNITS SHALL CONFORM TO ASTM C90-11, MORTAR SHALL BE TYPE-S, ASTM C270-12  
 5.8 MASONRY SHEAR WALLS SHALL BE REINFORCED AND GROUTED WITH ACI 530.1 GROUT AS SHOWN IN THE DRAWINGS.

### 6.0 STRUCTURAL STEEL

6.1 ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM SPECIFICATION A992 GRADE 50 EXCEPT PIPE COLUMNS WHICH SHALL CONFORM TO ASTM A53, AND TUBE COLUMNS TO ASTM A500, LATEST EDITIONS. ANCHOR BOLTS SHALL BE A307 STEEL AND MISCELLANEOUS EMBEDDED ITEMS SHALL BE A36 STEEL.  
 6.2 ALL STRUCTURAL STEEL SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH THE LATEST PROVISIONS OF AISC "STEEL CONSTRUCTION MANUAL". USE A325 BOLTS AND E60XX-SMAW WELD ROD FOR CONNECTIONS UNLESS OTHERWISE NOTED.  
 6.3 UNLESS OTHERWISE NOTED, USE STANDARD BEAM SEAT CONNECTIONS WITH STEEL BOLTS OR WELDED EQUIVALENT. SELECT CONNECTIONS AT EACH SPICE TO SUPPORT 60% THE TOTAL UNIFORM LOAD CAPACITY FOR EACH GIVEN BEAM AND SPAN. USE A325 BOLTS UNLESS OTHERWISE NOTED.  
 6.4 MINIMUM WELDS TO BE BY AISC AND/OR AWS BUT NOT LESS THAN 3/16" CONTINUOUS FILLET UNLESS OTHERWISE NOTED.  
 6.5 PROVIDE ANCHORS FOR BEAMS BEARING ON MASONRY WHERE ANCHOR BOLTS ARE NOT SPECIFIED. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR NAILER HOLES, SPECIAL DETAILS AND MISCELLANEOUS STEEL.  
 6.6 PROVIDE TEMPORARY BRACING AND PRECAUTIONS NECESSARY TO WITHSTAND ALL CONSTRUCTION AND/OR WIND LOADS UNTIL ALL FIELD CONNECTIONS ARE COMPLETED AND SHEAR WALLS AND DECKS ARE IN PLACE.  
 6.7 STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH STEEL JOIST INSTITUTE SPECIFICATIONS AND SHALL BE OF THE TYPE SHOWN IN THE DRAWINGS.  
 6.8 PROVIDED COMPRESSION FLANGE BRACING TO ALL PURLINS, GIRTS AND SIMILAR FRAMING MEMBERS AT MIDSPAN ON MEMBERS UP TO 20 FT AND AT 1/3 POINTS ON MEMBERS BETWEEN 20 FT AND 30 FT.

### 7.0 METAL DECK

7.1 DECK SHALL BE GALVANIZED STEEL G90 AND OF THE TYPE CALLED FOR ON THE FRAMING PLANS AND SHALL BE CONTINUOUS WHERE POSSIBLE. STEEL SHALL CONFORM TO ASTM A611  
 7.2 DECKING SHALL BE INSTALLED AND ALL OPENINGS IN DECKS CUT AND REINFORCED IN ACCORDANCE WITH MANUFACTURER'S STANDARD DETAILS AND SDI SPECIFICATIONS.  
 7.3 WELDED REQUIREMENTS SHALL BE AS FOLLOWS, UNLESS OTHERWISE SHOWN IN THE DRAWINGS:  
 7.3.1 LONGITUDINAL JOINTS BETWEEN ADJACENT ROOF DECKS SHALL BE FASTENED TOGETHER WITH WELDS AT 12"  
 7.3.2 ATTACH FLOOR DECK (PERMANENT CONCRETE FORM) TO EACH INTERMEDIATE SUPPORT AT 12" O.C. WITH PUDDLE WELD. EACH END LAP TO BE WELDED AT 10" O.C. EACH SIDE LAP IS WELDED AT EACH END PLUS TWO INTERMEDIATE WELDS (4 WELDS PER SHEET PER SIDE). USE 5/8" PUDDLE WELD AND WASHERS.

### 8.0 LIGHT - GAGE METAL FRAMING

8.1 LIGHT-GAGE METAL FRAMING SHALL BE GALVANIZED G90 ACCORDING TO ASTM A653-11 AND OF THE SIZE AND GAGE SHOWN ON THE DRAWINGS.  
 8.2 FRAMING ROLLED FROM STEEL - 18 GAGE AND LIGHTER SHALL CONFORM TO ASTM A653, GRADE A, WITH A MINIMUM YIELD STRESS OF 33,000 PSI.  
 8.3 FRAMING FROM STEEL - GAGE 16 AND HEAVIER SHALL CONFORM TO ASTM A653, GRADE D WITH A MINIMUM YIELD STRESS OF 56,000 PSI.

### 9.0 DRY PACKING AND NON-SHRINK GROUT

9.1 DRY PACK OR NON-SHRINK GROUT SHALL BE PROVIDED AS FOLLOWS:  
 9.1.1 BETWEEN COLUMNS BASES AND FOOTINGS  
 9.1.2 MATERIAL SHALL MEET CRD-C 621 AND BE NON-METALLIC  
 9.1.3 CURE MATERIAL 3-DAYS MINIMUM, BEFORE APPLICATION OF ANY SUPERIMPOSED LOADS.

### 10.0 TIMBER

10.1 ALL STRUCTURAL TIMBER SHALL BE PRESSURE TREATED SOUTHERN YELLOW PINE NO. 1 OR BETTER UNLESS SHOWN OTHERWISE. MINIMUM BENDING STRESS 1200 PSI, MODULUS OF ELASTICITY 1600 KSI AND SERVICE MOISTURE CONTENT OF NO MORE THAN 19%.  
 10.2 PROVIDE FULL DEPTH BLOCKING BETWEEN JOISTS AT 8 FT CENTERS MAXIMUM FOR ALL FLOOR SPANS OVER 8 FT.

### 11.0 GENERAL

11.1 IF ANY CONDITIONS VARY FROM THE DRAWINGS OR THE CONTRACTOR REQUIRES FURTHER CLARIFICATION, ASK THE ARCHITECT AND SITE ENGINEER TO VERIFY. DO NOT PROCEED W/ ANY WORK WITHOUT AGREEMENT BETWEEN THE ARCHITECT AND SITE ENGINEER.  
 11.2 ENGINEER'S APPROVAL MUST BE SECURED FOR ALL STRUCTURAL SUBSTITUTIONS.  
 11.3 PRIOR TO INSTALLATION OF MECHANICAL AND ELECTRICAL EQUIPMENT OR OTHER ITEMS TO BE ATTACHED TO THE STRUCTURE, ENGINEER'S APPROVAL OF CONNECTIONS AND SUPPORTS SHALL BE OBTAINED. UNLESS OTHERWISE SPECIFICALLY DETAILED ON ARCHITECTURAL AND STRUCTURAL DRAWINGS, RESPECTIVE SUBCONTRACTOR SHALL FURNISH ALL HANGERS, CONNECTIONS, ETC. REQUIRED FOR INSTALLATIONS OF HIS ITEMS.  
 11.4 PROVIDE ALL EMBEDDED ITEMS IN STRUCTURE AS NOTED ON ARCHITECTURAL, MECHANICAL, ELECTRICAL AND STRUCTURAL DRAWINGS. MISCELLANEOUS EMBEDDED ITEMS AND ANCHOR BOLTS SHALL BE FURNISH BY STEEL SUPPLIER AND INSTALLED BY CONTRACTOR.  
 11.5 PROVIDE ASPHALTIC MASTIC COATINGS ON ALL STEEL EXPOSED TO THE EARTH.  
 11.6 SUBMIT SHOP AND ERECTION DRAWINGS FOR ALL REINFORCING, STEEL JOIST, STRUCTURAL STEEL AND METAL DECK TO ENGINEER FOR WRITTEN APPROVAL. THE MANUFACTURE OR FABRICATION OF ANY ITEMS PRIOR TO WRITTEN APPROVAL OF SHOP DRAWINGS SHALL BE AT ENTIRELY THE RISK OF THE CONTRACTOR.  
 11.7 ALL MAJOR STEEL AND STEEL JOIST SHOP DRAWINGS SUBMITTED SHALL INCLUDE CALCULATIONS AND BEAR THE STAMP OF A REGISTERED PROFESSIONAL ENGINEER.  
 11.8 CONTRACTOR TO VERIFY ALL DIMENSIONS BEFORE PROCEEDING WITH ANY WORK.  
 11.9 PLACE ALL CONDUIT AND SERVICES UNDER ALL SLABS ON GRADE. WHERE CONDUIT AND SERVICES ARE IN BEAMS, COLUMNS AND SUSPENDED SLABS ENSURE THAT MINIMUM COVER OF 1-1/2" TO ALL REINFORCEMENT IS MAINTAINED. DO NOT BUNDLE CONDUIT IN SLABS.  
 11.10 TYPICAL LINTELS:  
 SMALL SPAN LINTELS ARE DEFINED AS HORIZONTAL SIMPLY SUPPORTED ELEMENTS SUPPORTING NO MORE THAN 4-FT OF 8" CMU MASONRY IN AN INTERIOR OR EXTERIOR AXIAL CONDITION AND NOT SUBJECT TO TORSION OR ASYMMETRIC LOADING.

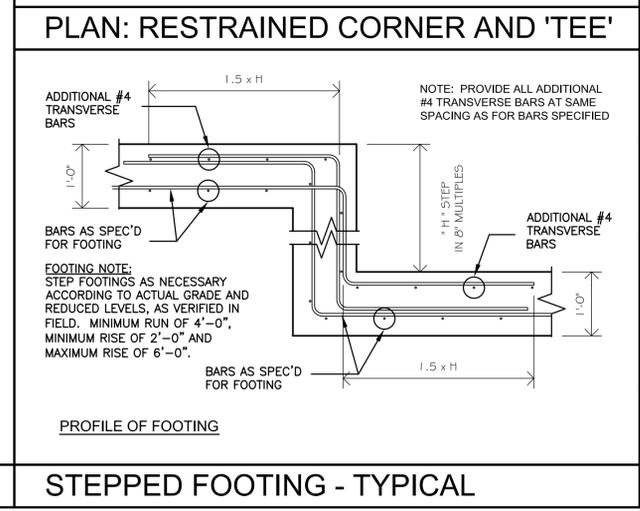
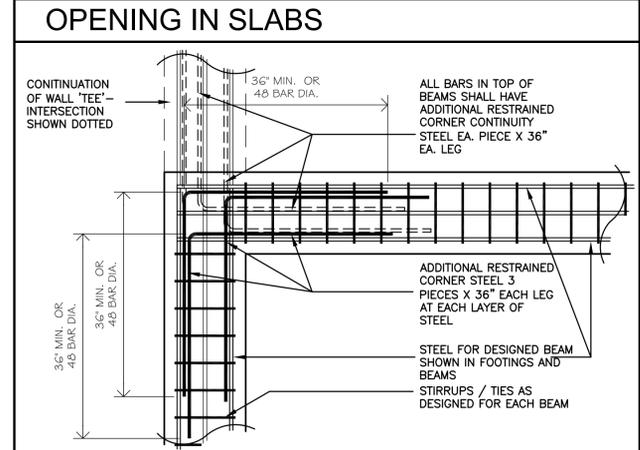
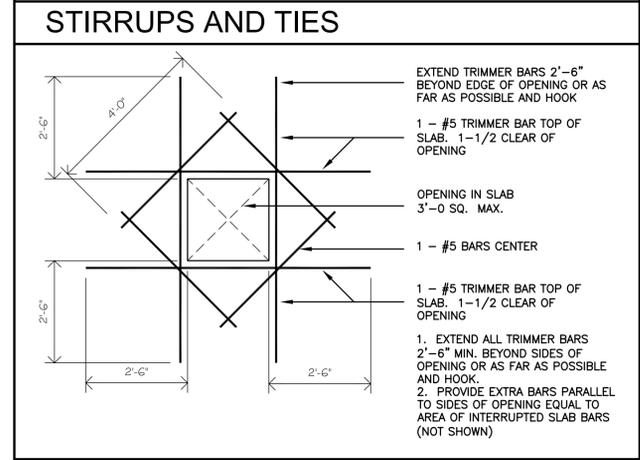
### 11.11 REINFORCEMENT LAYOUT DESIGNATION

### ABBREVIATIONS AND SYMBOLS

4-#5	REBAR DESIGNATION IN A BEAM. QTY = 4 NO. BARS, #5 = 5/8" (INCH) DIAMETER
C-02	COLUMN REFERENCE NUMBER
CONC.	CONCRETE
DET.#	DETAIL REFERENCE NUMBER
DIA.	DIAMETER
EA.	EACH
E.W.	EACH WAY
F-04	FOOTING REFERENCE NUMBER
HORIZ.	HORIZONTAL
O.C.	ON CENTER
QTY	QUANTITY
REINF.	REINFORCING OR REINFORCEMENT
SSL :-	STRUCTURAL SLAB LEVEL
TEMP. STEEL	TEMPERATURE AND SHRINKAGE STEEL
VERT.	VERTICAL
W/	WITH
W12X256	WIDE FLANGE BEAM. STRUCTURAL BEAM
WWF.	WELDED WIRE FABRIC = STRUCTURAL MESH

### 4

BAR SIZE	D	90° HOOK		135° HOOK		SEISMIC STIRRUP/TIE	
		A OR G	H	A OR G	H	135° HOOK	H
# 3	1-1/2"	4"	3"	4-1/4"	3"		
# 4	2"	4-1/2"	4"	4-1/2"	3"		
# 5	2-1/2"	6"	5-1/2"	5-1/2"	3-3/4"		
# 6	4-1/4"	12"	8"	8"	4-1/2"		
# 7	5-1/4"	14"	9"	9"	5-1/4"		
# 8	6"	16"	10-1/2"	10-1/2"	6"		



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02	DO NOT SCALE DRAWINGS
03	

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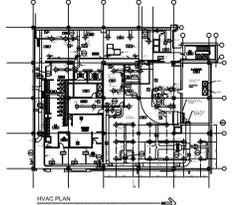
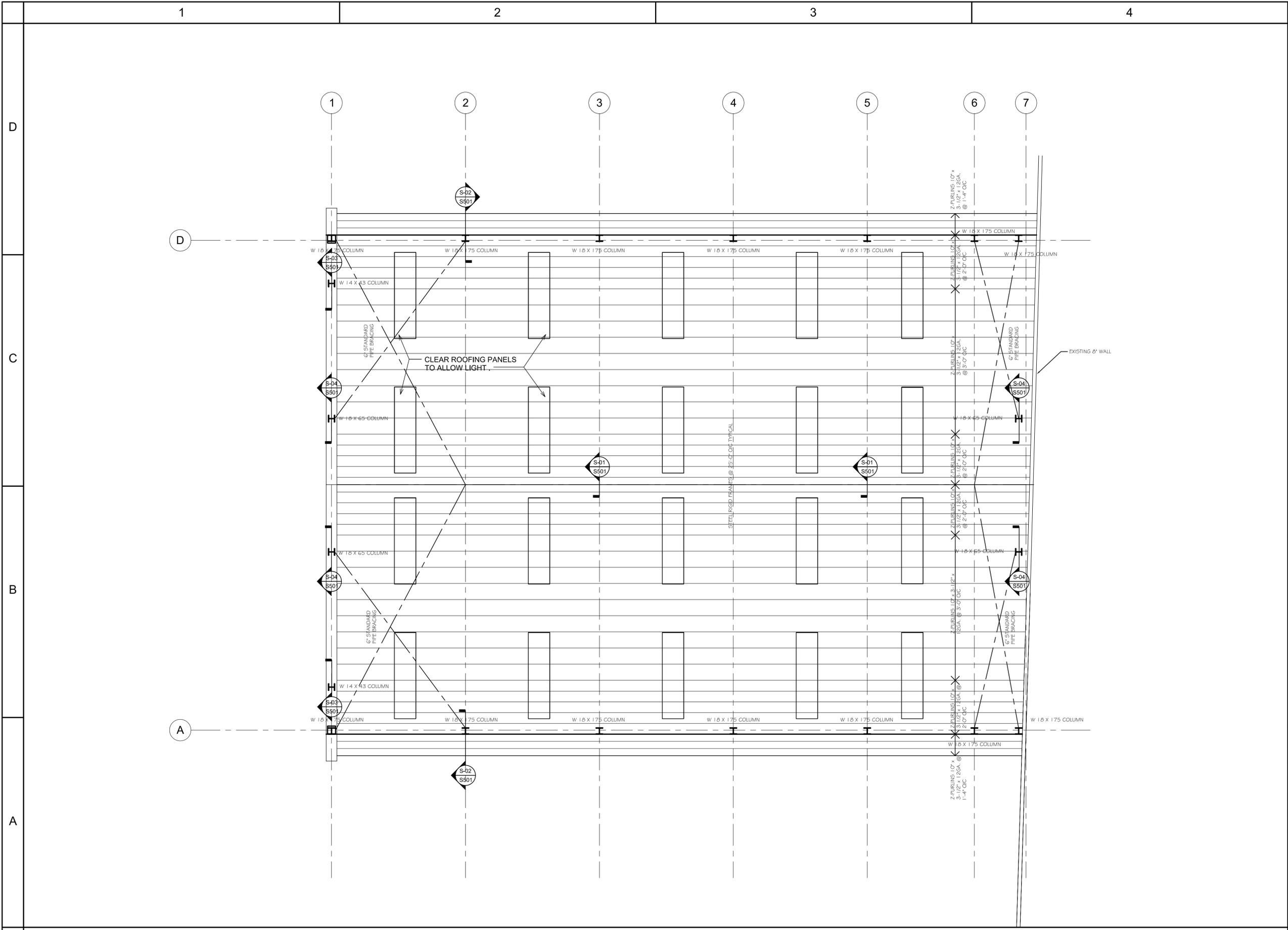
PRELIMINARY SET - AUG '12  
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REV #	DESCRIPTION	DWN	YYMMDD
B	REVISED ISSUE TO ARCH.	MT	120823
A	PRELIMINARY ISSUE TO ARCH	MT	120712

ISSUE INFORMATION.	<b>B</b>
DESIGNER:	MT
DRAWN:	MT
JOB REF:	ARCH
DATE:	12 07 12
SCALE:	AS NOTED
DWG NAME:	<b>STRUCTURAL NOTES</b>
DWG NO:	<b>S-001</b>
	VERS. 6.0 - 110411







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REV #	DESCRIPTION	DWN	YYMMDD

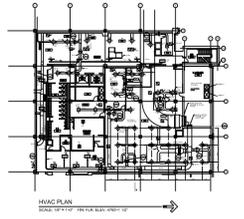
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DESIGNER:		MT
DRAWN:		MT
JOB REF:		--
DATE:		12 07 12
SCALE:		AS NOTED

DWG NAME:  
**GA ROOF PLAN**

DWG NO:  
**S-103**

GA - ROOF PLAN

SCALE : 1/8" = 1'-0"



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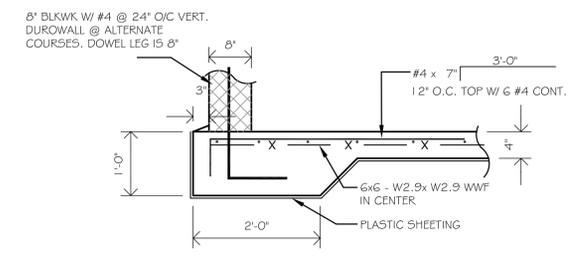
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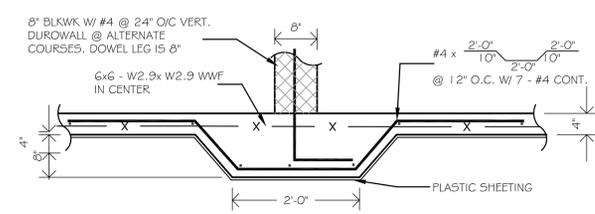
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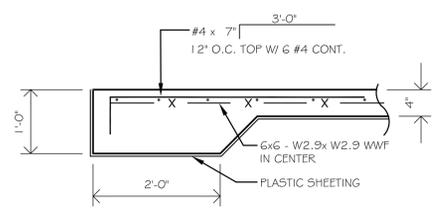
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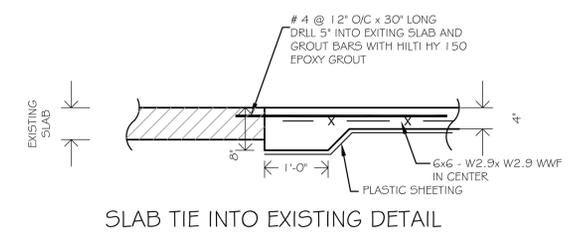
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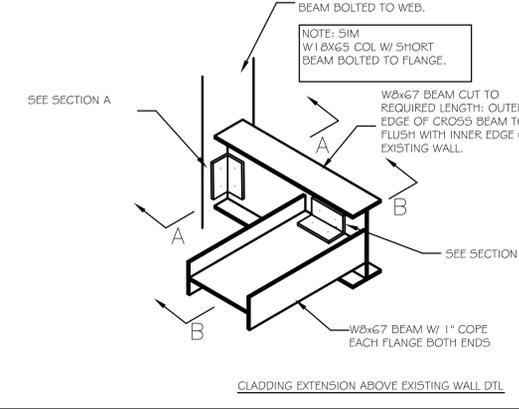
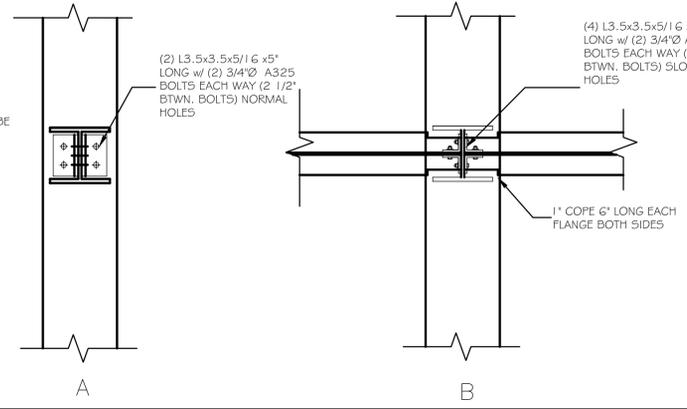
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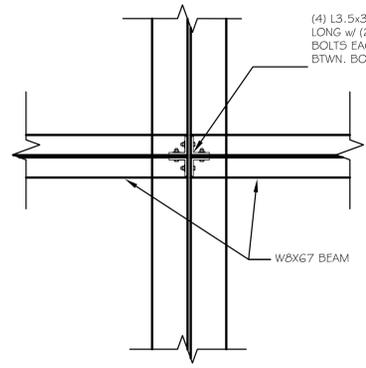
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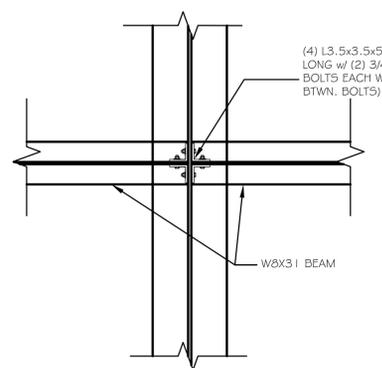
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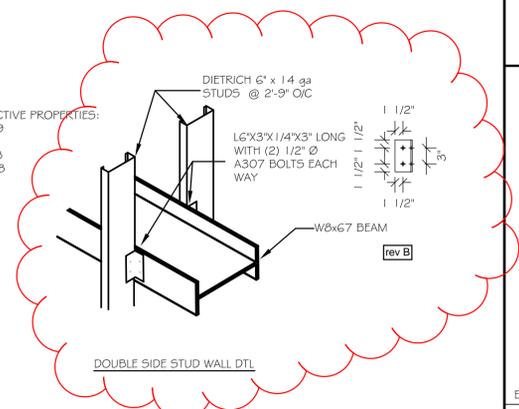
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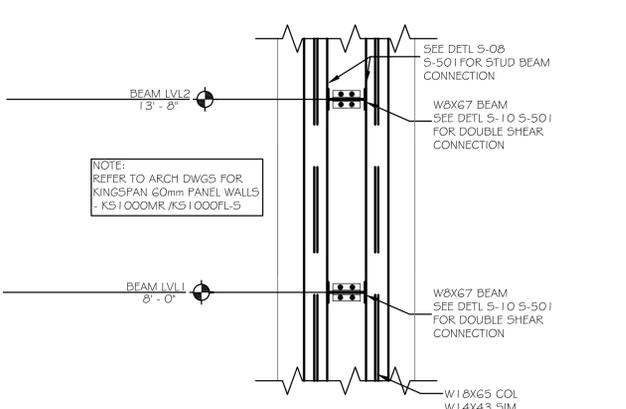
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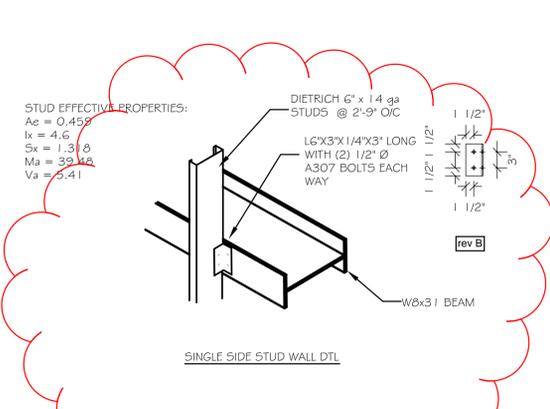
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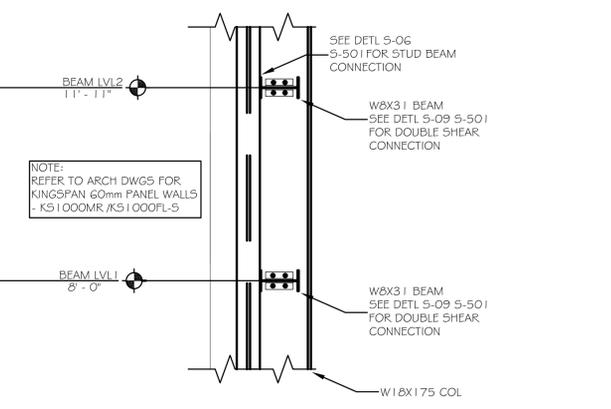
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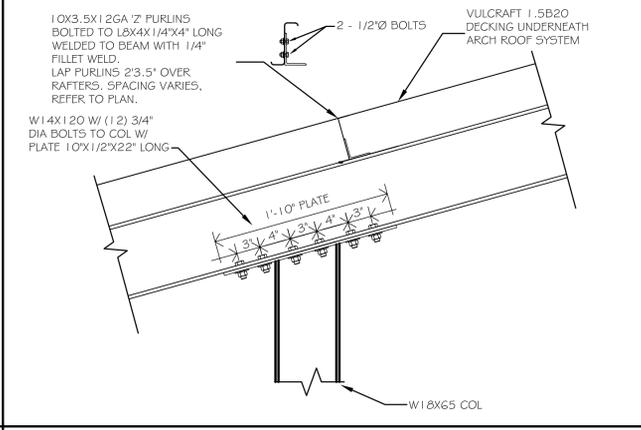
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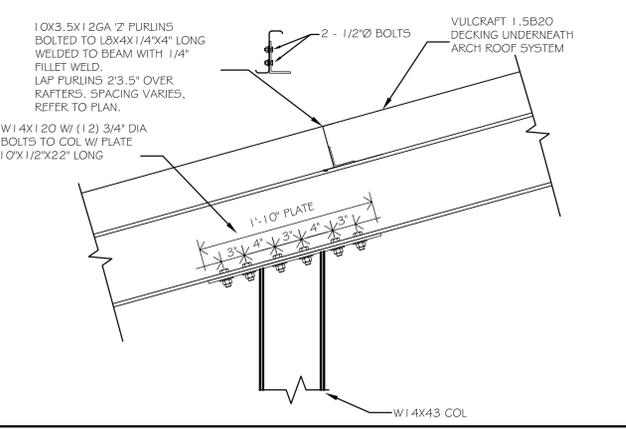
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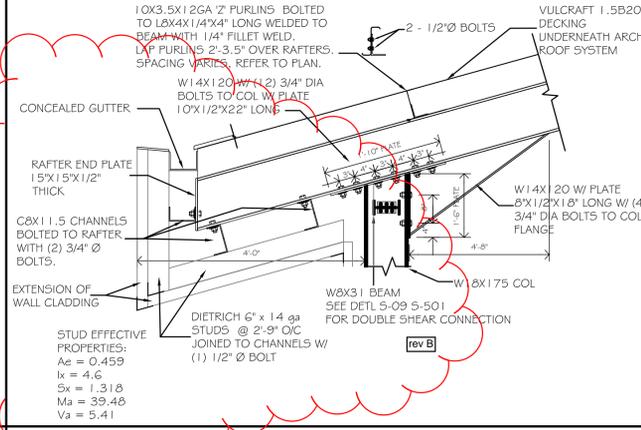
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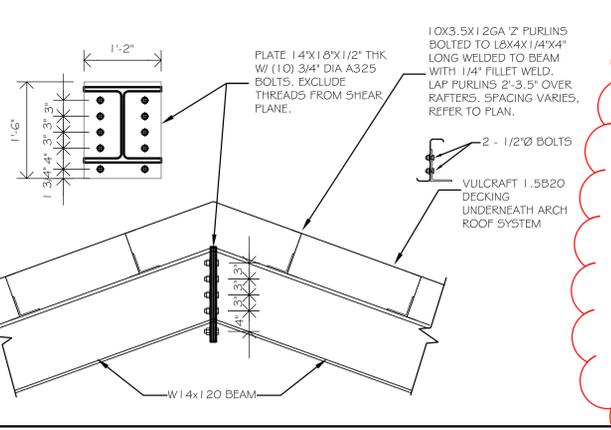
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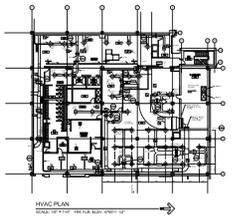
S-03 SCALE: 3/4"=1'-0"



S-02 SCALE: 1/2"=1'-0"



S-01 SCALE: 3/4"=1'-0"



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ISSUE INFORMATION.

DESIGNER:	MT
DRAWN:	MT
JOB REF:	--
DATE:	12 07 12
SCALE:	AS NOTED
DWG NAME:	DETAILS-02
DWG NO:	S-502

